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Buckling of Ship Grillages - Part II

by

D. A. Danielson D. P. Kihl

September 1997

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Prepared for:

Naval Postgraduate Monterey, CA 93943-5000 19971103 038

and

Naval Surface Warfare Center Carderock Division Washington, DC 20084-5000

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The report was prepared by:

Donald Danielson

Professor

Department of Mathematics

Reviewed and Released by:

iv not woods

W. Max Woods

Chairman

Mathematics Department

David W. Netzer

Associate Provost and Dean of Research

REPORT DOCUMENTATION PAGE

NSN 7540-01-280-5500

Form Approved
OMB No. 0704-0188

1997

Public reporting burden for this collection of information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden. to Washington Headquarters Services, Directorate for information Operations and Reports, 1215 Jefferson Davis Highway, Suite 1204, Arlington, VA 22202-4302, and to the Office of Management and Budget, Paperwork Reduction Project (0704-0188), Washington, DC 20503.

1. AGENCY USE ONLY (Leave blank)) 2. REPORT DATE		AND DATES COVERED
September 1997		Technical Re	<u>port October 1996 - Septembe</u>
4. TITLE AND SUBTITLE			5. FUNDING NUMBERS
BUCKLING OF SHIP GRIL	LAGES - PART II		NOO167-96-WR-60226
6. AUTHOR(S)			┥
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D. A. Danielson			i
D. P. Kihl			1
7. PERFORMING ORGANIZATION NAM	ME(S) AND ADDRESS(ES)		8. PERFORMING ORGANIZATION
Naval Postgraduate Scho	ool Naval Surface	Warfare Center	
Code MA/Dd	Code 653		NPS-MA-97-005
Monterey, CA 93943	Carderock Divi		
	Bethesda, MD	20084-5000	
9. SPONSORING/MONITORING AGEN	CV NAME(S) AND ADDRESS(ES)		10. SPONSORING / MONITORING
Naval Surface Warfare		aduate School	AGENCY REPORT NUMBER
Carderock Division	Monterey, CA		, i
Washington, D.C. 2008		,	
washington, 2000			
11. SUPPLEMENTARY NOTES		C (1)	
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official policy or post	ition of the Departme	nt of Defense	or the US Government.
12a. DISTRIBUTION / AVAILABILITY ST			12b. DISTRIBUTION CODE
Approved for public re	lease; distribution i	s unimited.	
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13. ABSTRACT (Maximum 200 words)			
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The subject of the	is report is the mech	nanical behavio	or of stiffened plates,
basic structural compone	ents of ships and sub	marines. The	buckling loads of grillages
subjected to axial comp	ression with and with	out lateral pr	ressure are carcurated
using a finite element	based eigenvalue anai	ysis.	
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14. SUBJECT TERMS			13. NUMBER OF PAGES
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Snip Griffages, Flate	s, Buckling, Structu	res, Finite Ele	16. PRICE CODE
17. SECURITY CLASSIFICATION 18	8. SECURITY CLASSIFICATION	19. SECURITY CLASS	10. 11.11.2
			10. 11.11.2

BUCKLING OF SHIP GRILLAGES - PART II

D. A. Danielson Code MA/Dd Naval Postgraduate School Monterey, CA 93943

D. P. Kihl Code 653 Naval Surface Warfare Center, Carderock Division Bethesda, MD 20084-5000

August 1997

ABSTRACT

The subject of this report is the mechanical behavior of stiffened plates, basic structural components of ships and submarines. The buckling loads and modes of grillages subjected to axial compression without and with lateral pressure are calculated using finite element based eigenvalue analyses.

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INTRODUCTION

Stiffened plates are basic structural components of ships and submarines. In our earlier report*, we have calculated the axial buckling loads of grillages with the use of a well-known finite element code. In the present work, we continue the calculation of buckling loads for new grillages.

The grillages now modeled consist of 3 base plates, 2 longitudinal edge plates, 4 longitudinal stiffeners, and 2 transverse stiffeners. Two sets of grillages (I and II) having different plating thicknesses and stiffener dimensions are considered (Figures 1, 2 and 12, 13). We define the following grillage dimensions, of which the first 4 are common to all grillages:

a1 = length between transverse stiffeners = 96''

a2 = length between transverse stiffener and grillage end = 96''

b1 = width between longitudinal stiffeners = 27''

b2 = width between outer longitudinal stiffener and grillage side = 7.5''

b3 = width of longitudinal edge plates

t1 = thickness of inner base plate and longitudinal edge plates

t2 = thickness of outer base plates

dw1 = depth of longitudinal webs

tw1 = thickness of longitudinal webs

df1 = depth of longitudinal flanges

tf1 = thickness of longitudinal flanges

dw2 = depth of transverse webs

tw2 = thickness of transverse webs

df2 = depth of transverse flanges

tf2 = thickness of transverse flanges

^{*}D. A. Danielson and D. P. Kihl, "Buckling of Ship Grillages," Naval Postgraduate School Technical Report NPS-MA-96-002, September 1996.

In these grillages, the outer base plates also have thickness t1 in regions of width 6" adjacent to the transverse stiffeners. The material is isotropic steel with Young's modulus $E = 3 \times 10^7$ psi and Poisson's ratio $\nu = .3$. For grillages I, the base plates and longitudinal edge plates are HSLA steel with yield stress 85 ksi, whereas the longitudinal and transverse stiffeners are HS steel with yield stress 60 ksi. For grillages II, the steel types are interchanged: the plates are HS steel, whereas the stiffeners are HSLA steel.

All grillages have the same imposed boundary conditions:

- (1) One end of a grillage has all 3 displacement components zero and all 3 rotation components zero.
- (2) The other end of the grillage where the force is applied has uniform axial displacement with the other 2 displacement components zero and all 3 rotation components zero.
- (3) The ends of the transverse stiffeners have vertical displacement zero but the other 2 displacement components and all 3 rotation components are free.
- (4) For the pressure cases, the longitudinal edge plates are also restrained vertically at 10.5" intervals along the outer sides.
- (5) All other nodes on the plates edges are completely free.

To each of these grillages we apply compressive axial loading together with one of 3 normal loads: no pressure, an initial pressure loading of 10 psi (simulating fluid on the plating side), or an initial pressure loading of -10 psi (simulating fluid on the stiffener side). Furthermore, to assess the effects of the longitudinal edge plates, we consider both uncut and cut edge plates. This makes a total of 12 different cases to study.

The finite element code used is MSC Nastran/Patran. The pre-processing and post-processing is done with Patran (version 6.2). The buckling analysis (solution 105) is done with Nastran (version 69). The meshing is done entirely with Quad 4 plate elements. Note that this is a full finite element model of the entire grillage and does not assume symmetry. We use linear geometry and linear material properties for all cases. Computations are performed on a Silicon Graphics Indy Workstation.

GRILLAGES I*

The first set of grillages has the following dimensions:

b3 = 6''

t1 = .4375''

t2 = .5''

dw1 = 6''

tw1 = .17''

df1 = 4''

tf1 = .215''

dw2 = 14''

tw2 = .255''

df2 = 5''

tf2 = .42''

The mesh size used is:

mesh length of all elements, mesh width of base plate elements = 1.5'' mesh width of web and edge plate elements = 1'' mesh width of flange elements = .5''

^{*}Subsequent to the completion of the grillages I cases, it was discovered that the longitudinal edge plates should have extended 4" below the base plating (instead of 4" on the stiffeners' side). For the unpressurized uncut case, the buckling force is then 1860 kips (as opposed to 1865 kips). Since the difference between the two results is so small, we did not correct the grillages I cases. (In the grillages II cases, the longitudinal edge plates are exactly centered along the base plating.) In addition, in the positively pressurized grillages I cases, the longitudinal edge plates should have been constrained vertically along the base plating's side rather than the stiffeners' side. For the positively pressurized cut case, the buckling force is then 1350 kips (as opposed to 1352 kips). Since the difference between the two results is so small, and to ensure the end displacement under pressure -10 psi is the negative of the end displacement under pressure 10 psi, we did not change the side along which the constraints are applied. (In the pressurized grillages II cases, the longitudinal edge plates are constrained vertically along the base plating's side.)

NO PRESSURE

First we consider the linear response under an axial compressive force of 1865 kips. The prebuckling state is one of uniform axial compression with little bending. The magnitude of the maximum stress occurs in the thinner regions of width 6" in the outer base plates (Figure 3). Note that the magnitude of the maximum stress is about 50 ksi, considerably less than the yield stress.

The bifurcation buckling force is 1865 kips. The buckling mode is a deformation involving primarily bending with little stretching (Figure 4). The inner base plate buckles into a square quilt of half-wavelength 24". The adjacent webs also buckle with half-wavelength 24". The flanges mainly just rotate about their center line. The edge plates also just rotate about their line of attachment to the base plate. In our earlier report we called this buckling mode TRIPPING.

To assess the effects of the longitudinal edge plates, we next cut the element sides lying along vertical lines at intervals 10.5". The force required to buckle this grillage with 48 edge cuts is 1721 kips (8% less than the uncut case). The buckling mode is similar but with relatively larger deformations near the cut edges (Figure 5).

POSITIVE PRESSURE (Fluid on Plating Side)

Next we consider the linear response under a normal pressure loading of 10 psi and an axial compressive force of 1436 kips. The prebuckling state now involves axial compression and bowing out of the entire grillage with lateral torsional bending of the stiffeners (Figure 6). However, the magnitude of the stress is still less than the yield stress, except at the ends of the central longitudinal stiffeners.

In the presence of positive pressure, the bifurcation buckling force is reduced to 1436 kips (23% less than the unpressurized case). The buckling mode is similar to the unpressurized case (Figure 7).

To again assess the effects of the longitudinal edges, we again make 48 vertical cuts in

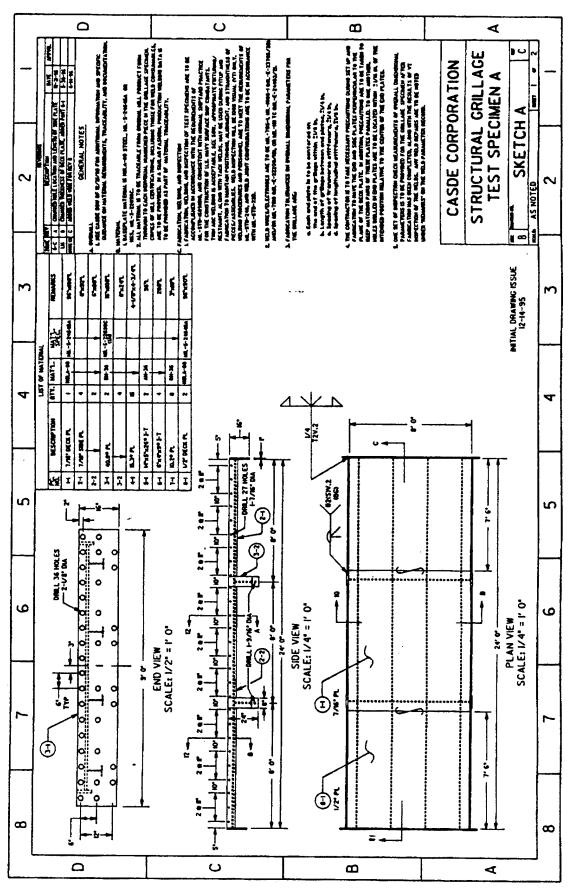
the edge plates. This cut grillage buckles at an axial compressive force of 1352 kips (6% less than the uncut pressurized case). Since the edge plates are now restrained vertically along their upper edges at each of the cuts, the buckling mode does not show as much deformation near the cut edges as the cut unpressurized case (Figure 8).

NEGATIVE PRESSURE (Fluid on Stiffener Side)

Finally we consider the linear response under a normal pressure loading of -10 psi and an axial compressive force of 1516 kips. Note that the prebuckling compressive stress in the center of the longitudinal stiffeners is now larger than in the base plates (Figure 9). The magnitude of the stress is now less than the yield stress everywhere.

In the presence of negative pressure, the bifurcation buckling force is 1516 kips (19% less than the unpressurized case). The buckling mode now has half-wavelength 48" (Figure 10).

The cut grillage under negative pressure buckles at an axial compressive force of 1427 kips (Figure 11) (again 6% less than the uncut pressurized case).



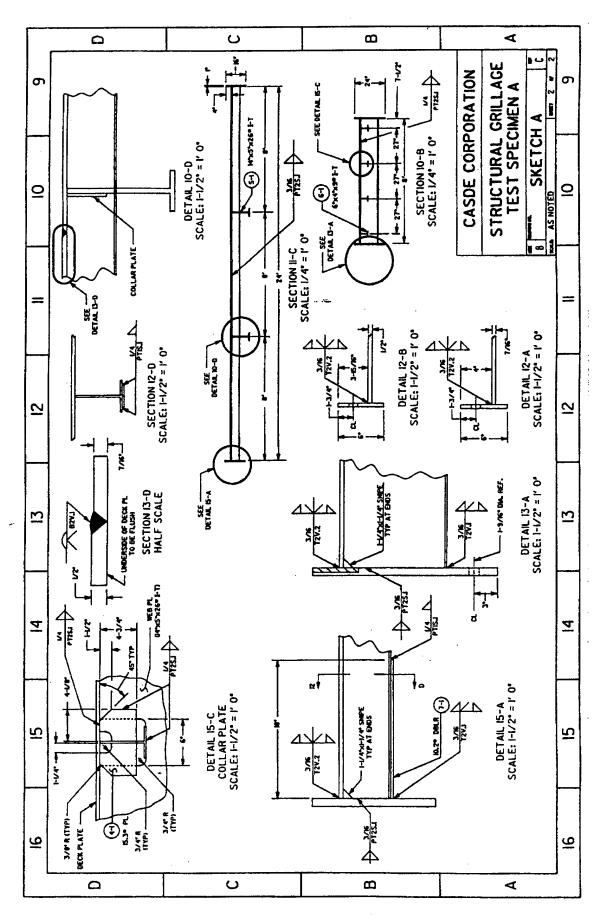
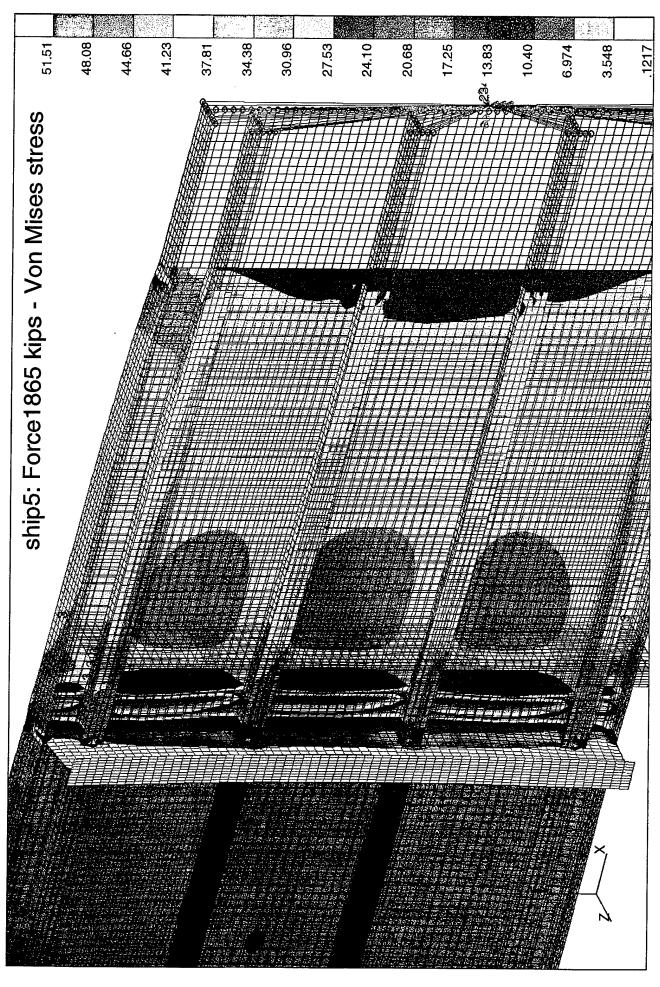


Figure 2



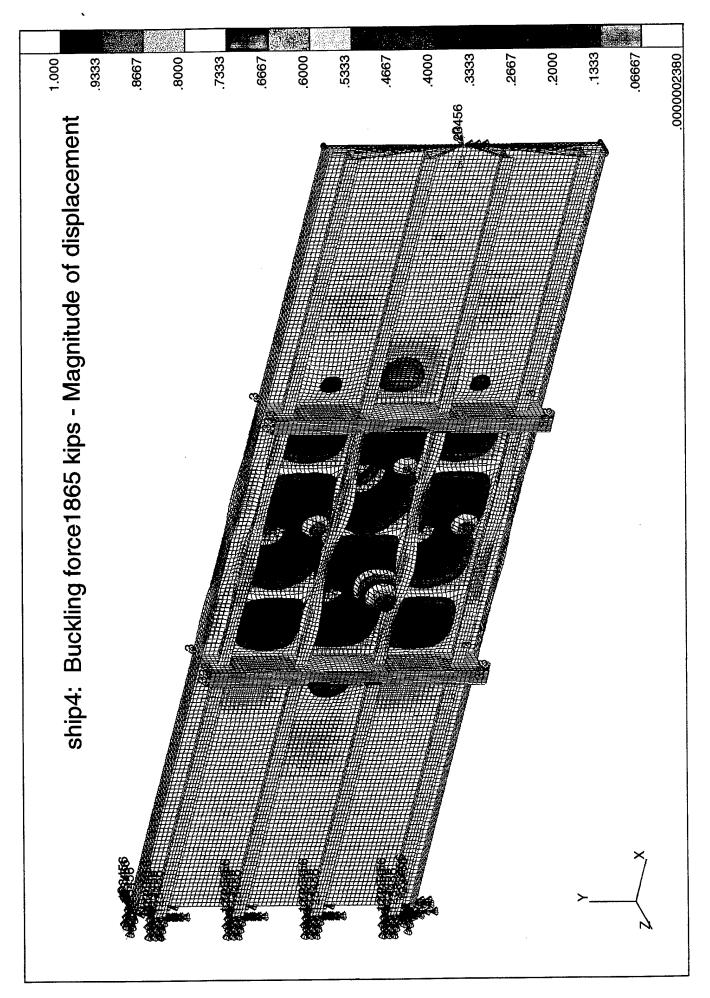


Figure 4

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Figure 7

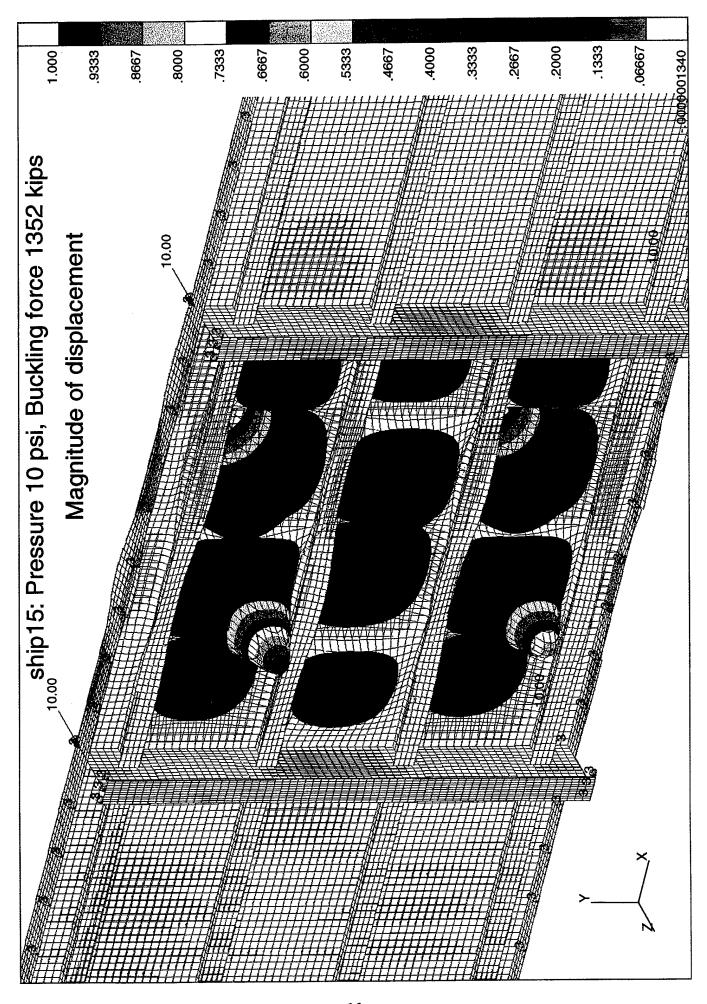


Figure 9

Figure 10

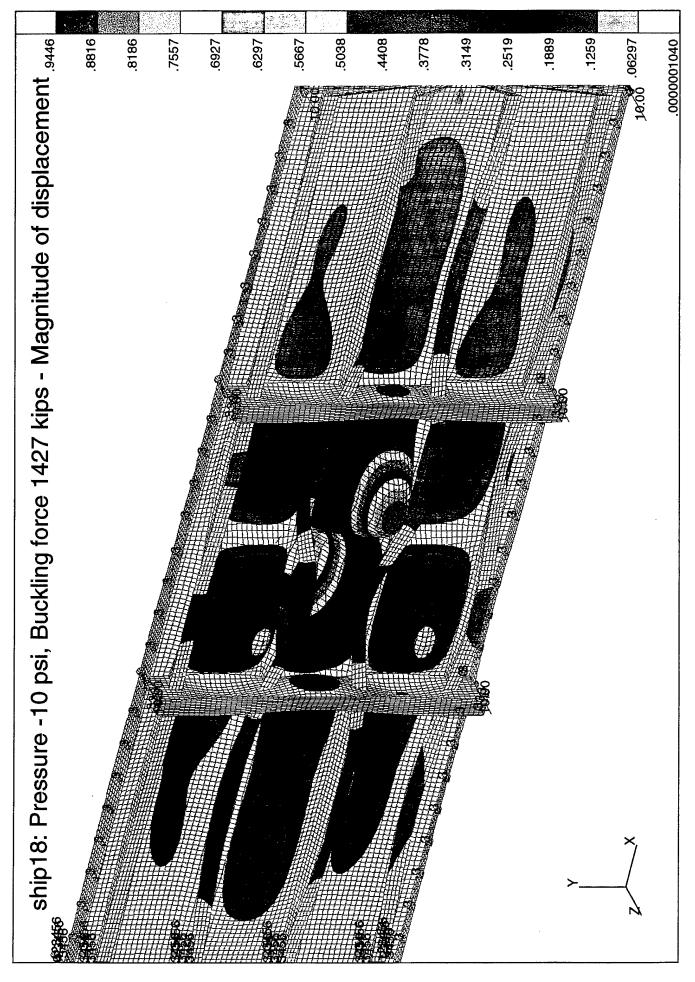


Figure 11

GRILLAGES II

The second set of grillages has the following dimensions:

b3 = 8.5''

t1 = .375''

t2 = .4375''

 $\mathrm{dw1} = 5.5''$

 $tw1=.3125^{\prime\prime}$

 $\mathrm{df1} = 4.25''$

tf1=.3125''

 $\mathrm{dw2} = 13.5''$

 $\mathrm{tw}2=.25''$

df2=5''

tf2 = .4375''

The mesh size now used is:

mesh length of all elements, mesh width of base plate elements = 1.5''

mesh width of edge plate elements = .85''.

mesh width of webs = 5''.

mesh width of longitudinal flanges = .53125''

mesh width of transverse flanges = .625''

NO PRESSURE

The magnitude of the maximum stress in the prebuckling state under an axial force of 1552 kips is about 35 ksi, much less than the yield stress.

The bifurcation buckling force is 1552 kips for the uncut grillage and 1406 kips for the cut grillage. The buckling modes are similar to the corresponding grillages I cases (Figures 14 and 15).

POSITIVE PRESSURE (Fluid on Plating Side)

The magnitude of the maximum stress in the prebuckling state under a normal pressure of 10 psi and an axial compressive force of 1403 kips is about 46 ksi, still considerably less than the yield stress.

In the presence of positive pressure, the bifurcation buckling force is 1403 kips for the uncut grillage and 1293 kips for the cut grillage. The buckling modes are similar to the corresponding grillages I cases (Figures 16 and 17).

NEGATIVE PRESSURE (Fluid on Stiffener Side)

In the presence of negative pressure, the bifurcation buckling force is 1685 kips for the uncut grillage and 1555 kips for the cut grillage. The buckling mode now has half-wavelength of about 20" (Figures 18 and 19).

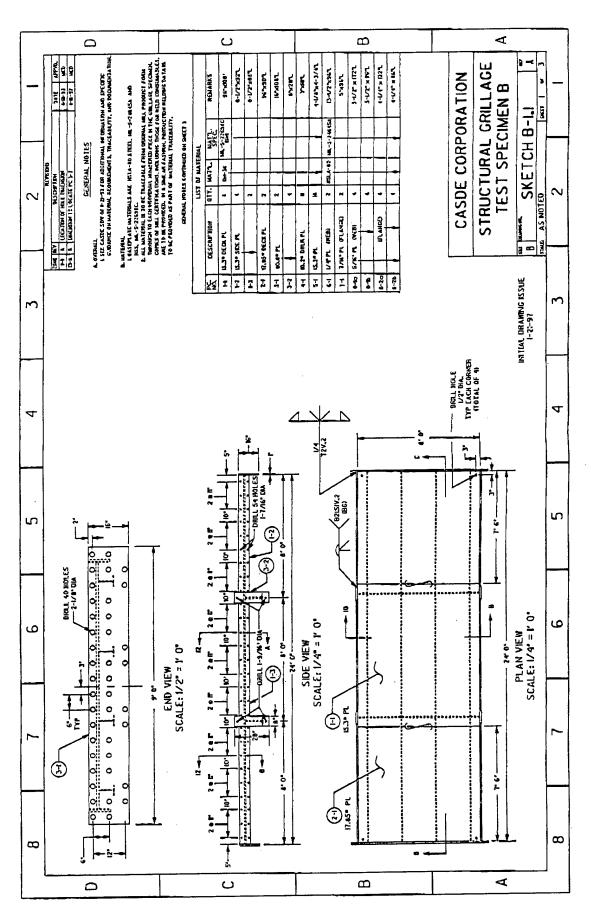


Figure 12

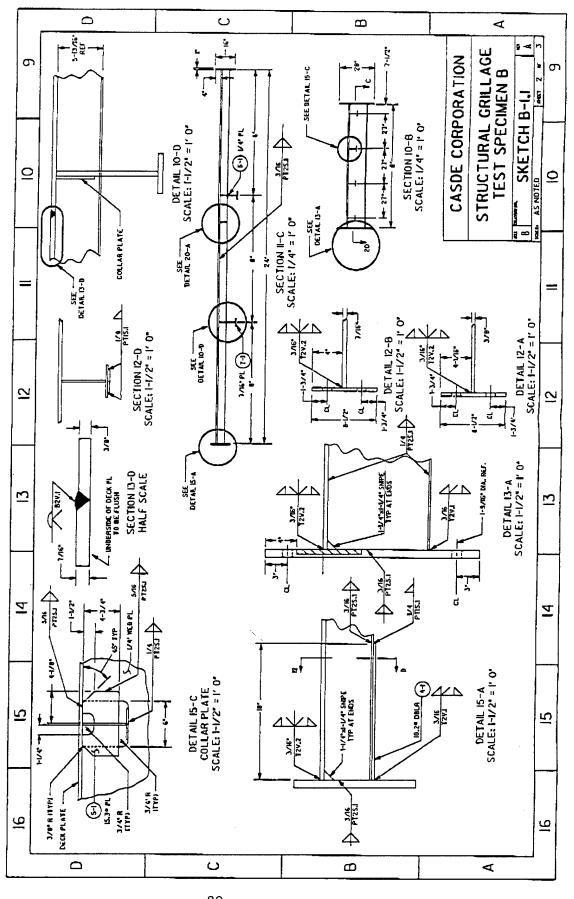


Figure 14

Figure 15

Figure 16

Figure 17

Figure 18

Figure 19

CONCLUSIONS

GRILLAGES I

	Continuous Edges	Cut Edges
Pressure 0	3047", 1865 kips	2963", 1721 kips
Force 0	.001643", 10 psi	.001758", 10 psi

TABLE 1: END DISPLACEMENT, FORCE OR PRESSURE

	Continuous Edges	Cut Edges
Pressure 0	$1865~\mathrm{kips}$, 1	1721 kips , .923
Pressure 10 psi	1436 kips , .770	1352 kips , .725
Pressure -10 psi	1516 kips , .813	1427 kips , .765

TABLE 2: COMPRESSIVE BUCKING FORCE, NORMALIZED BUCKLING FORCE

GRILLAGES II

	Continuous Edges	Cut Edges
Pressure 0	2542", 1552 kips	2477" , 1406 kips
Force 0	.001938", 10 psi	.002130" , 10 psi

TABLE 3: END DISPLACEMENT, FORCE OR PRESSURE

	Continuous Edges	Cut Edges
Pressure 0	1552 kips , 1	1406 kips , .906
Pressure 10 psi	1403 kips , .904	1293 kips , .833
Pressure -10 psi	1685 kips , 1.086	1555 kips , 1.002

TABLE 4: COMPRESSIVE BUCKING FORCE, NORMALIZED BUCKLING FORCE

The axial displacement at the end of a grillage under the compressive buckling force or the positive pressure is given in Tables 1 or 3. Since the prebuckling state is linear, the axial displacement under both loads is the linear superposition of the two displacements in a column. To get the displacement at any other load, we simply multiply by the displacement/load ratio obtained from Tables 1 or 3.

The predicted buckling loads for the 12 cases are summarized in Tables 2 and 4. Dividing each force by the upper left entry in the table, we obtain the normalized buckling force, from which we can immediately see the reduction in buckling force due to applying pressure or/and cutting the edge plates.

Judging from our earlier work, the buckling loads we have now calculated are likely to be greater than collapse loads to be measured in future experiments. Unmodeled factors which would tend to decrease the buckling loads include geometric imperfections, residual stress, and relaxed boundary conditions. On the other hand, we have calculated only the forces required to initiate buckling, but the grillages are likely to have significant postbuckling strength before ultimate collapse.

ACKNOWLEDGEMENTS

The authors were supported by the Naval Surface Warfare Center, Carderock Division, and by the Naval Postgraduate School Research Program.

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